ADDENDUM NO. 1 March 14, 2024

# Stantec

The following items represent changes in the drawings and/or Project Manual, and Contractors shall be governed accordingly.

**RE: PET Scan Addition to BRCC** 

**Baton Rouge Cardiology Center** 

FROM: Stantec Architecture Inc.

1200 Brickyard Lane, Suite 400 Baton Rouge, Louisiana 70802

TO: PROSPECTIVE BIDDERS

This addendum forms a part of the Construction Documents and modifies the original Bidding Documents dated February 27, 2024 as noted below. Acknowledge receipt of this Addendum in the space provided on the Bid Form. Failure to do so may subject the bidder to disqualification.

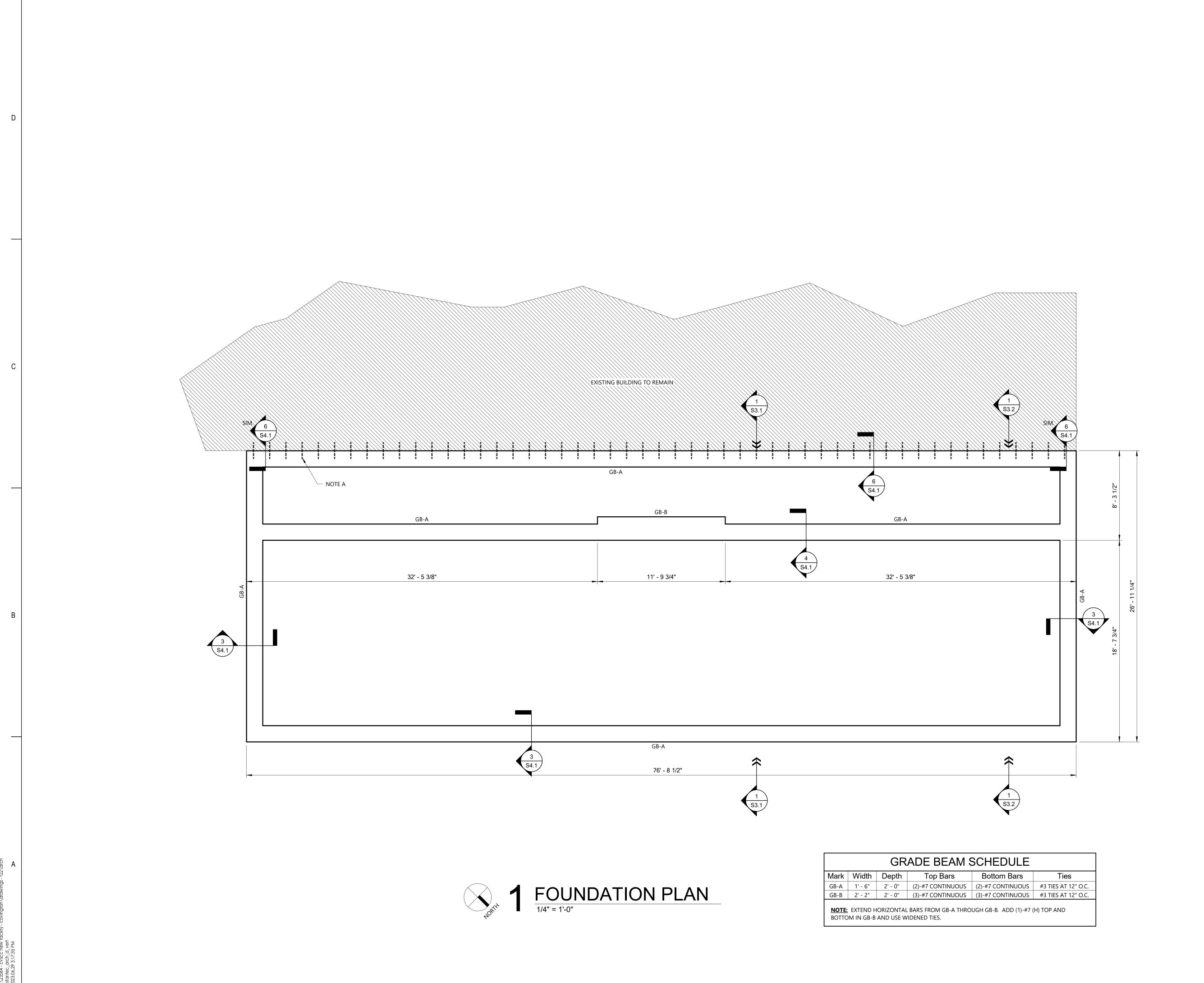
This Addendum consists of:

Page AD-1 (Structural items)
Drawings S1.1, S1.2, S2.1, S3.1, S3.2, S4.1, S5.1, S5.2, S5.3, S6.1, S6.2 (Structural Drawings)

#### **CHANGES TO STRUCTURAL**

1. Add attached Structural drawing sheets to Bid Documents.

**END OF ADDENDUM** 



FOUNDATIONS DETAILS.

AND PLACEMENT.

NOTE A:

SEE GENERAL NOTES FOR FORMING

BUILDING PAD REQUIREMENTS.

INTO EXISTING FOUNDATION.

REQUIREMENTS OF FOUNDATION ELEMENTS.

(2)-#4 DOWELS AT 18" O.C., (1) TOP AND (1) BOTTOM, 18" LENGTH, 9" EMBEDMENT IN EPOXY

SEE SPECIFICATION 31 2100 FOR EARTH MOVING AT

PROVIDE #5 L-BAR (a=10", b= 26") DOWELS AT 24" O.C. ALONG TOP OF ALL GRADE BEAMS. SEE

ALL GRADE BEAMS SHALL BE PLACED OVER A 2" THICK CONCRETE DRY BOTTOM AS DEEMED APPROPRIATE BY THE CONTRACTOR IF A PRECIPITATION EVENT IS ANTICIPATED BEFORE CONCRETE PLACEMENT. EXPOSED FOOTING AND GRADE BEAM BOTTOMS THAT DO NOT CONTAIN DRY BOTTOMS SHALL NOT BE SUBJECTED TO A PRECIPITATION EVENT PRIOR TO PLACING CONCRETE. THE GRADE BEAM AND SPREAD FOOTING SUBGRADE SHALL BE APPROVED BY THE TESTING AGENCY FOR ADEQUATE BEARING CAPACITY PRIOR TO PLACEMENT OF DRY

BOTTOMS/CONCRETE. DRY BOTTOMS/CONCRETE FOOTING SHALL BE PLACED AS SOON AS POSSIBLE AFTER APPROVAL AND NO PRECIPITATION EVENT SHALL OCCUR IN THE TIME BETWEEN APPROVAL

FOUNDATION PLAN NOTES AND LEGEND:

THE TOP OF ALL GRADE BEAMS AND PEDESTALS SHALL BE AT EL. -0'-8", UNLESS NOTED OTHERWISE.

Permit/Seal



PET Scan / BRCC

Project No.: 222706047

Addition to

 
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 FOUNDATION PLAN

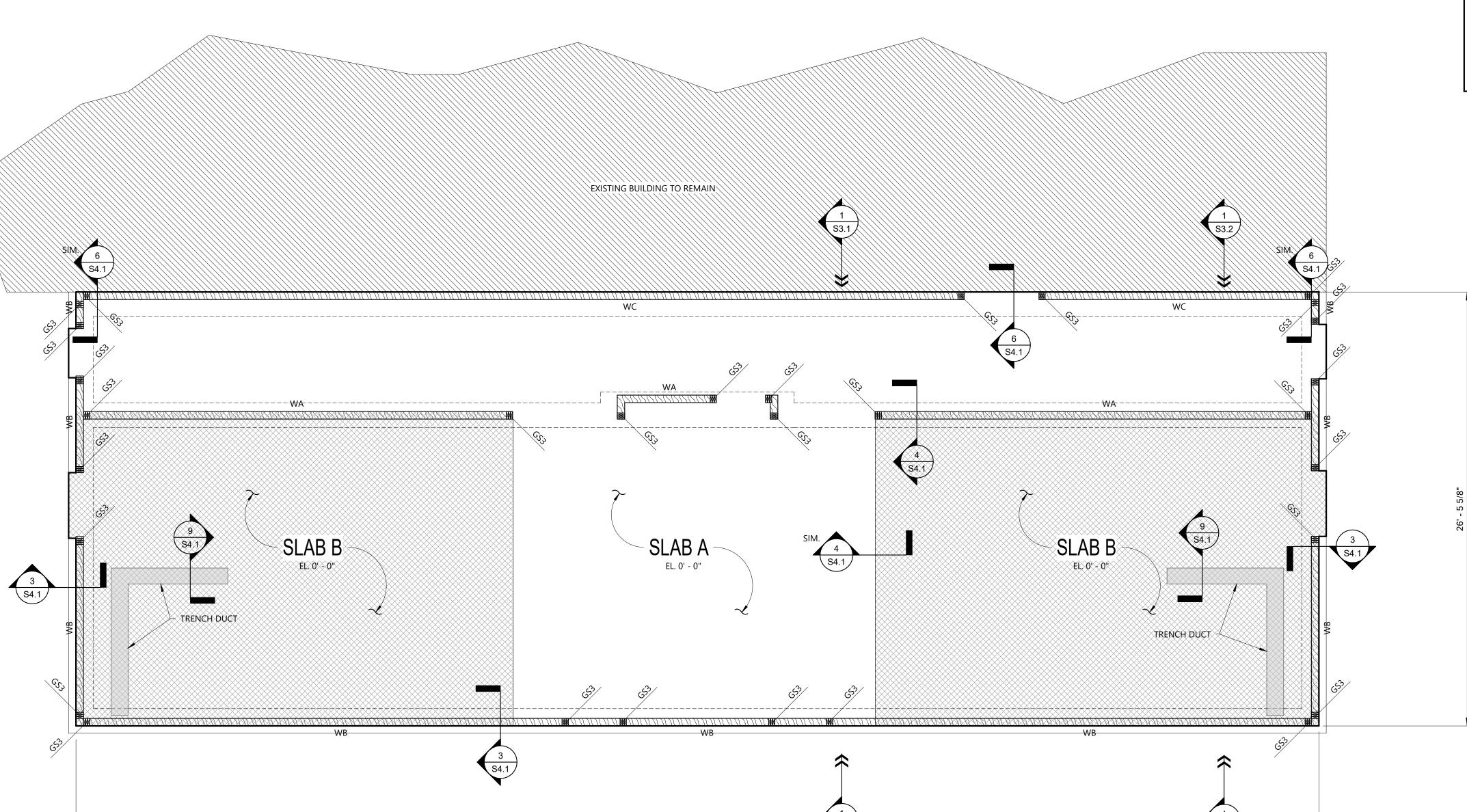
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#### **LOAD BEARING WALL NOTES:**

PROVIDE A CONTINUOUS TREATED 2x BOTTOM PLATE FOR ALL WALLS SAME SIZE AS WALL STUDS.

PROVIDE HORIZONTAL STUD BLOCKING AT 4'-0" O.C. MAX ALONG WALL HEIGHT (COINCIDE WITH SHEATHING EDGES WHERE APPLICABLE).

ANCHOR EXTERIOR WALLS TO SLAB WITH 5/8" DIA. GALVANIZED J-BOLTS (7" EMBED) AT 32" O.C. MAX. SPACING. USE SIMPSON GALVANIZED BP 5/8-3 PLATE WASHERS ON ALL J-BOLT ANCHORS. CENTER ANCHORS ON WIDTH OF BOTTOM PLATE. CENTER ANCHORS IN

ANCHOR INTERIOR WALLS TO SLAB WITH 5/8" DIA. GALVANIZED SIMPSON "TITENHD" CONCRETE SCREW ANCHORS AT 32" O.C. MAX. SPACING (4" EMBEDMENT INTO SLAB). USE SIMPSON GALVANIZED BP 5/8-3 PLATE WASHERS ON ALL SCREW ANCHORS. CENTER ANCHORS ON WIDTH OF BOTTOM PLATE. CENTER ANCHORS IN

PROVIDE J-BOLTS OR TITEN HD ANCHORS AT BOTH ENDS OF ALL BOTTOM PLATE PIECES.

PROVIDE METAL CLIPS, STRAPS, AND HOLD-DOWNS IN ACCORDANCE WITH ELEVATION 1/S5.3 FOR ALL LOAD BEARING WALLS SHOWN ON STRUCTURAL PLANS.

ALL STUD WALL FRAMING SHALL BE SOUTHERN PINE

VAPOR RETARDER WITH TAPED JOINTS ON 4" WWF 4x4 W4.0/W4.0 AND #4 BARS AT 48" O.C. EACH WAY. USE CONCRETE BLOCKS AT INSPECTED BY TESTING AGENCY AFTER PRIOR TO PLACEMENT OF DRAINAGE COURSE.

**SLAB B =** 8" THICK CONCRETE SLAB ON 15 MIL VAPOR RETARDER WITH TAPED JOINTS ON 4" GRAVEL ON COMPACTED FILL. REINFORCE WITH #4 BARS TOP AND BOTTOM AT 16" O.C. EACH WAY. SUBGRADE SHALL BE INSPECTED BY TESTING AGENCY AFTER COMPACTED FILL IS COMPLETE AND IMMEDIATELY PRIOR TO PLACEMENT OF DRAINAGE COURSE.

TRENCH DUCT = 12" WIDE x 3 1/2" DEEP. SEE ARCH. AND MEDICAL DRAWINGS FOR MORE INFORMATION.

GANGED STUD SCHEDULE METAL CLIP TO DETAIL TOP PLATE

#### **GANGED STUDS NOTES:**

GSx = PROVIDE(x)-KING STUDS. SAME SIZE AND GRADE AS WALL STUDS.

AND LOCATION OF DOOR AND WINDOW OPENINGS.

FOR SIMPSON HTT4 HOLDOWN USE 5/8" DIA. ASTM F1554 GRADE 55 GALVANIZED DRILL-AND-EPOXY ANCHOR RODS WITH 6" MIN. EMBEDMENT INTO SLAB USING SIMPSON SET-3G EPOXY. PROVIDE GALVANIZED HEAVY HEX

ALL METAL CLIPS/TIES AT TOP OF STUD SHALL ATTACH TO SAME SIDE OF TOP

PROVIDE GANGED STUDS (CHORD MEMBERS) AT EACH END OF SHEAR WALLS AS NOTED IN PLAN. CHORD MEMBERS ARE TO BE FULL HEIGHT. NUMBER OF STUDS NOTED DOES NOT INCLUDE REQUIRED JACK STUDS AT OPENING

**SLAB PLAN NOTES AND LEGEND:** 

SLAB A = 5" THICK CONCRETE SLAB ON 15 MIL GRAVEL ON COMPACTED FILL. REINFORCE WITH INTERSECTIONS OF #4 BARS TO KEEP WWF 1-1/2" CLEAR FROM TOP OF SLAB. SUBGRADE SHALL BE COMPACTED FILL IS COMPLETE AND IMMEDIATELY

SEE DETAIL 7/S4.1 FOR ADDITIONAL REINFORCEMENT AT SLAB OPENINGS.

HOLDDOWN HTT4 (2)-H2.5A

#### 2/S5.1 GS3 (3)-2x6

CONTRACTOR TO REFERENCE ARCHITECTURAL DRAWINGS FOR EXACT SIZE

NUT. FASTEN HOLDOWN TO GANGED STUDS WITH (26)-16d NAILS.

PLATES AS THE RAFTER/OUTLOOKER/TRUSS TIES.

LOCATIONS.

Permit/Seal



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SLAB PLAN

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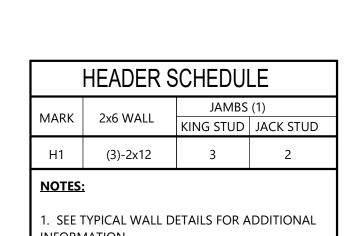
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1 SLAB PLAN

1/4" = 1'-0"

75' - 9 3/4"

S3.1



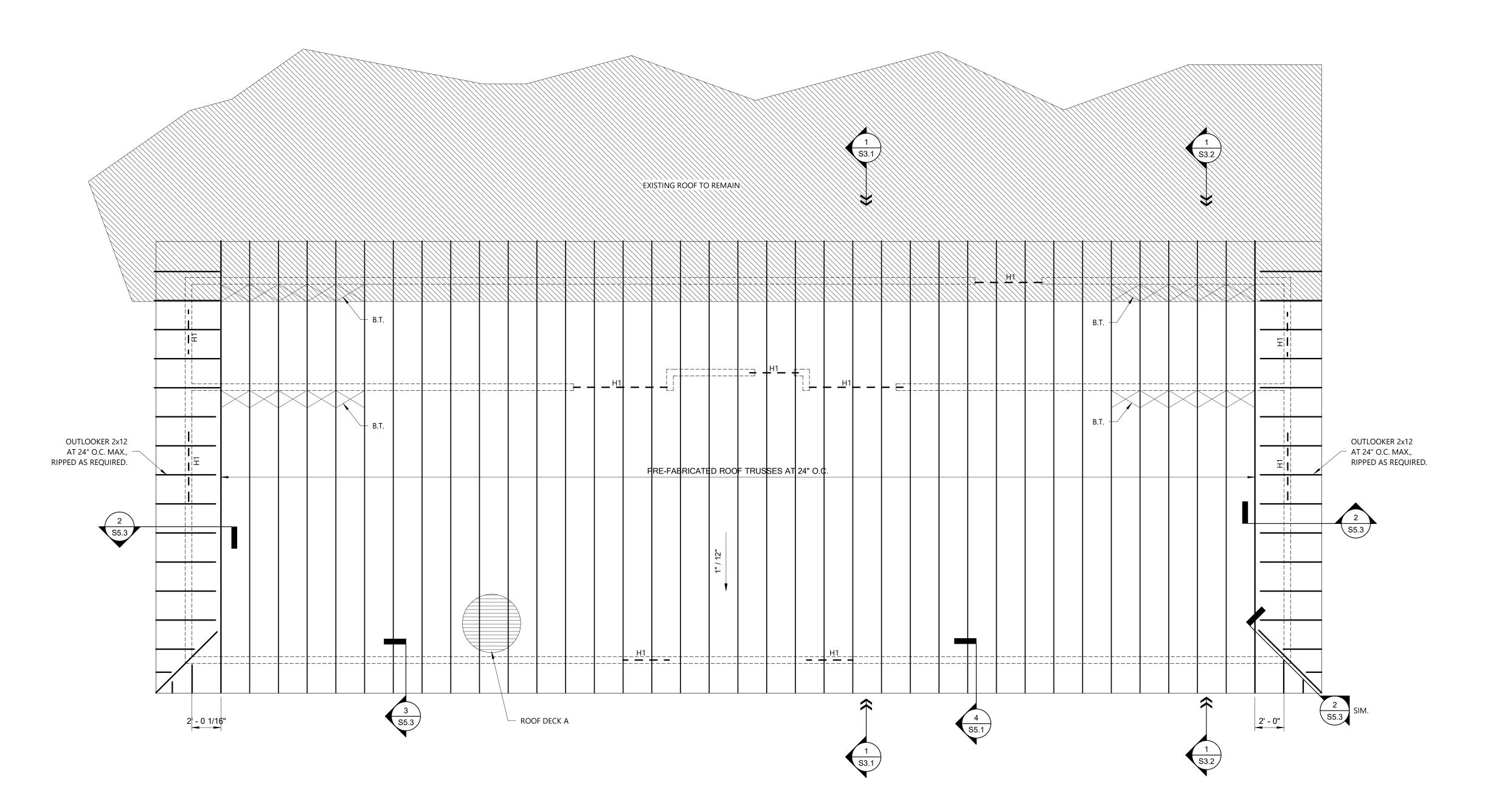
INFORMATION.

ROOF NOTES AND LEGEND:

**ROOF DECK A** = 3/4" APA RATED EXPOSURE 1 PLYWOOD SHEATHING ATTACHED PER SCHEDULE.

**H1** = HEADER - SEE SCHEDULE FOR INFO.

**B.T.** = PRE-ENGINEERED WOOD BLOCKING TRUSSES AT LOCATIONS INDICATED. EACH TRUSS SHALL BE DESIGNED TO TRANSFER ON ASD-FACTORED IN-PLANE HORIZONTAL FORCE IN EITHER DIRECTION ON 250 LBS. FROM THE ROOF DECK TO THE SUPPORT PLATE BELOW DUE TO WIND LOAD.



1 ROOF FRAMING PLAN

1/4" = 1'-0"

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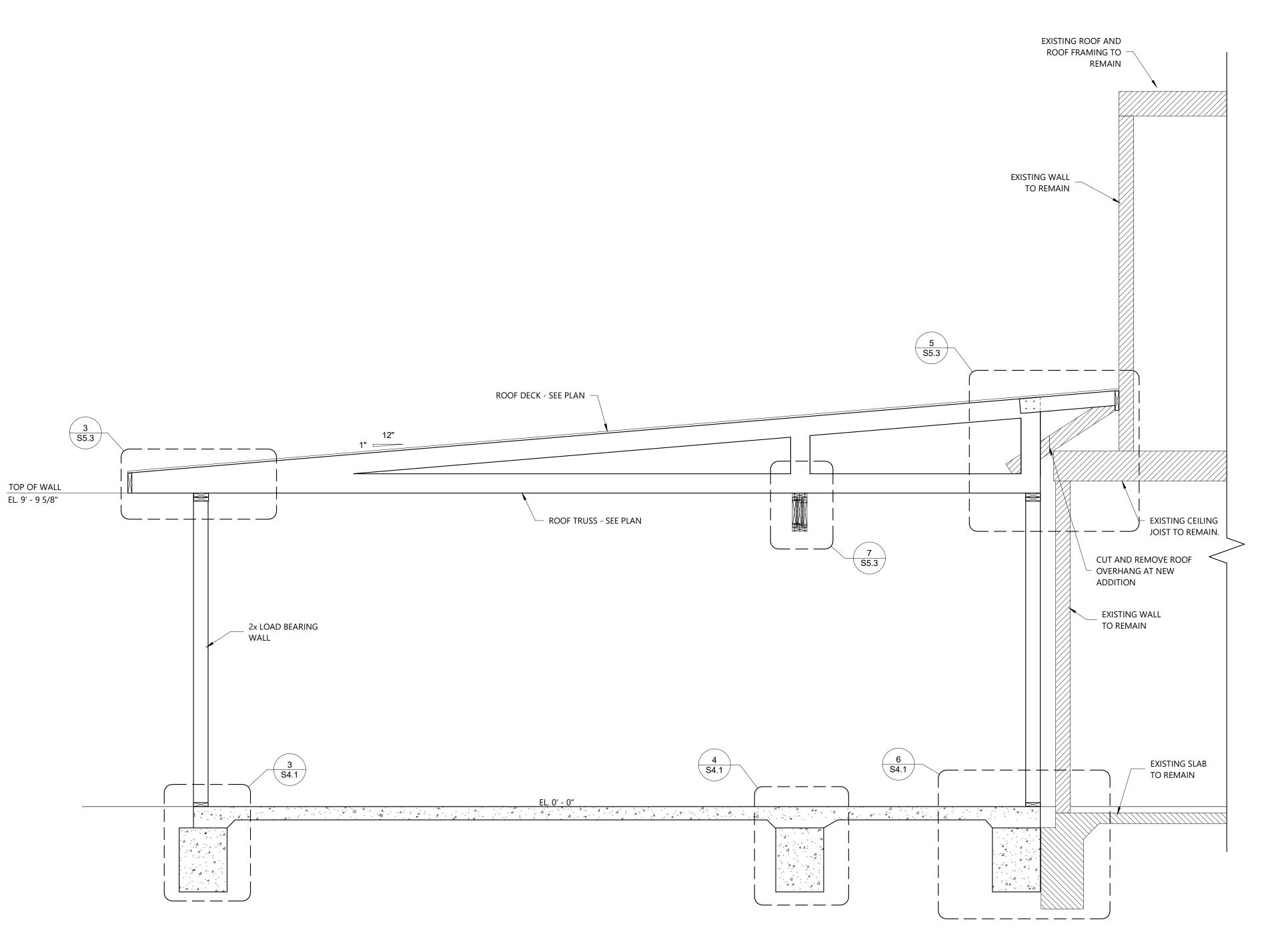
ROOF FRAMING PLAN

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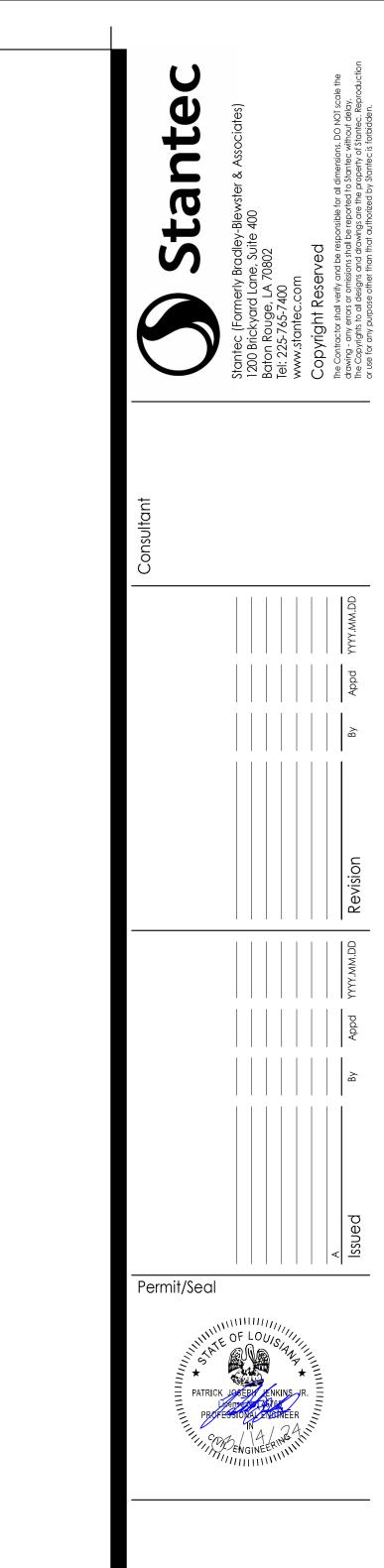
1 Building Section 1

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**BUILDING SECTION** 

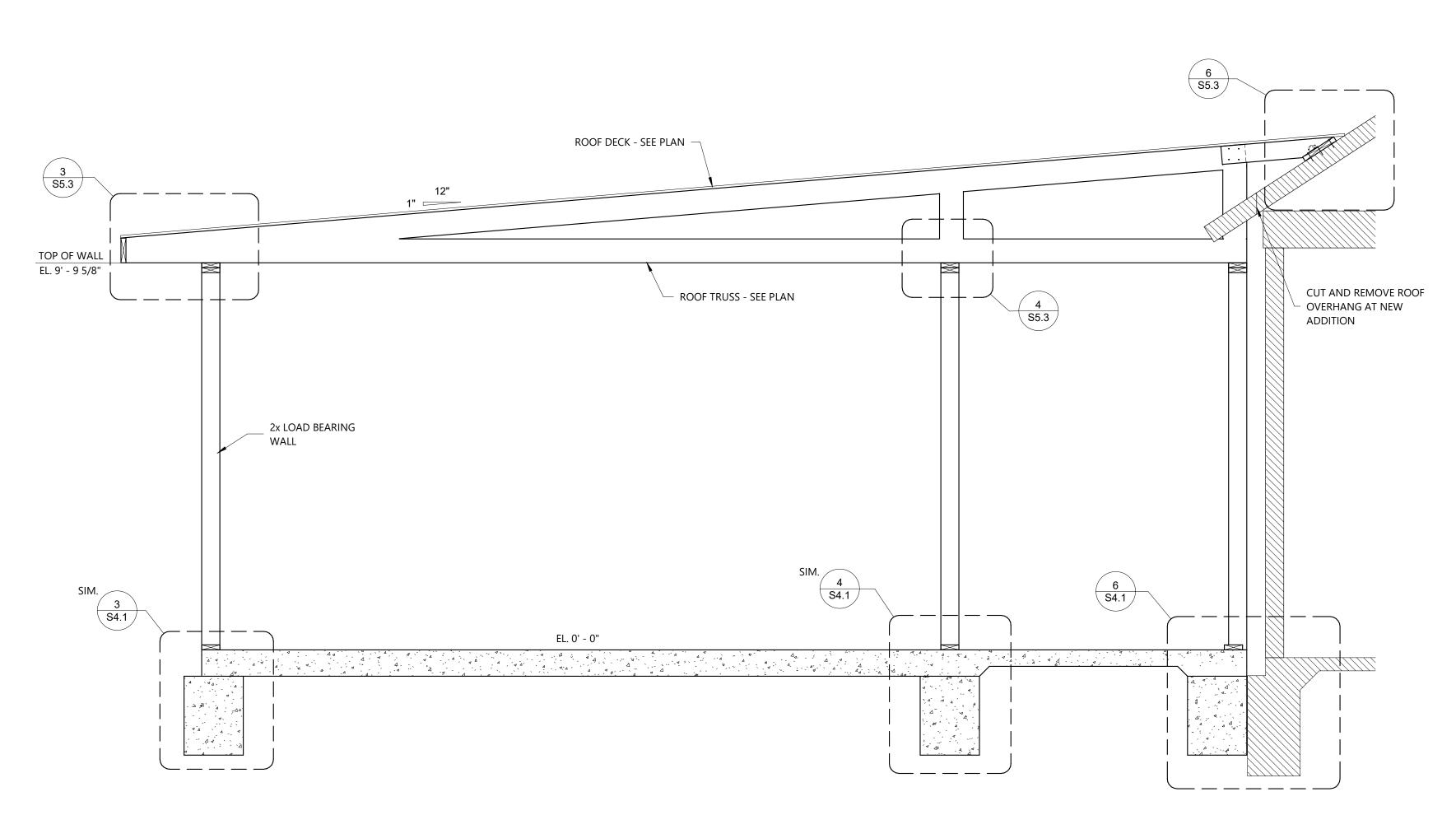
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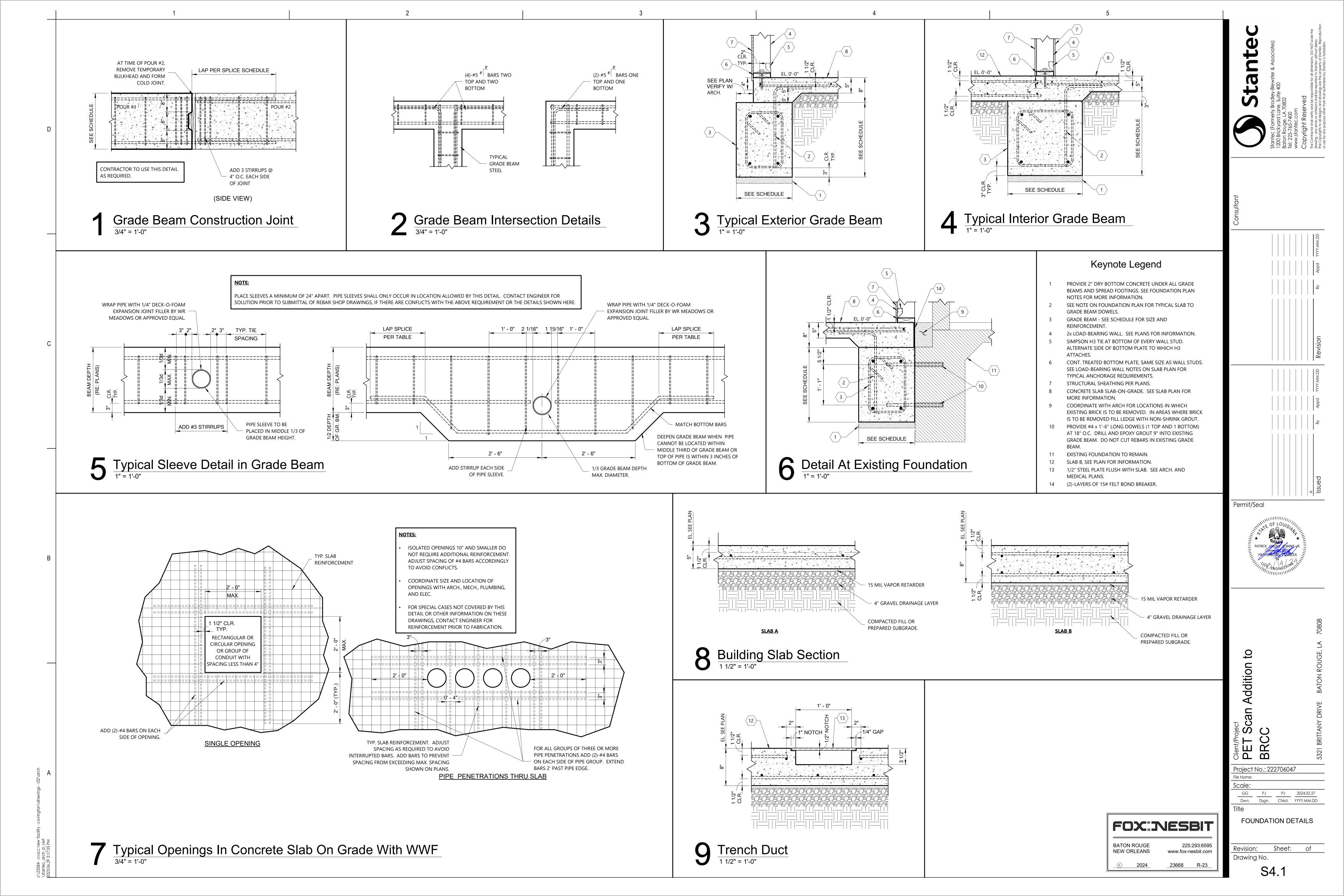
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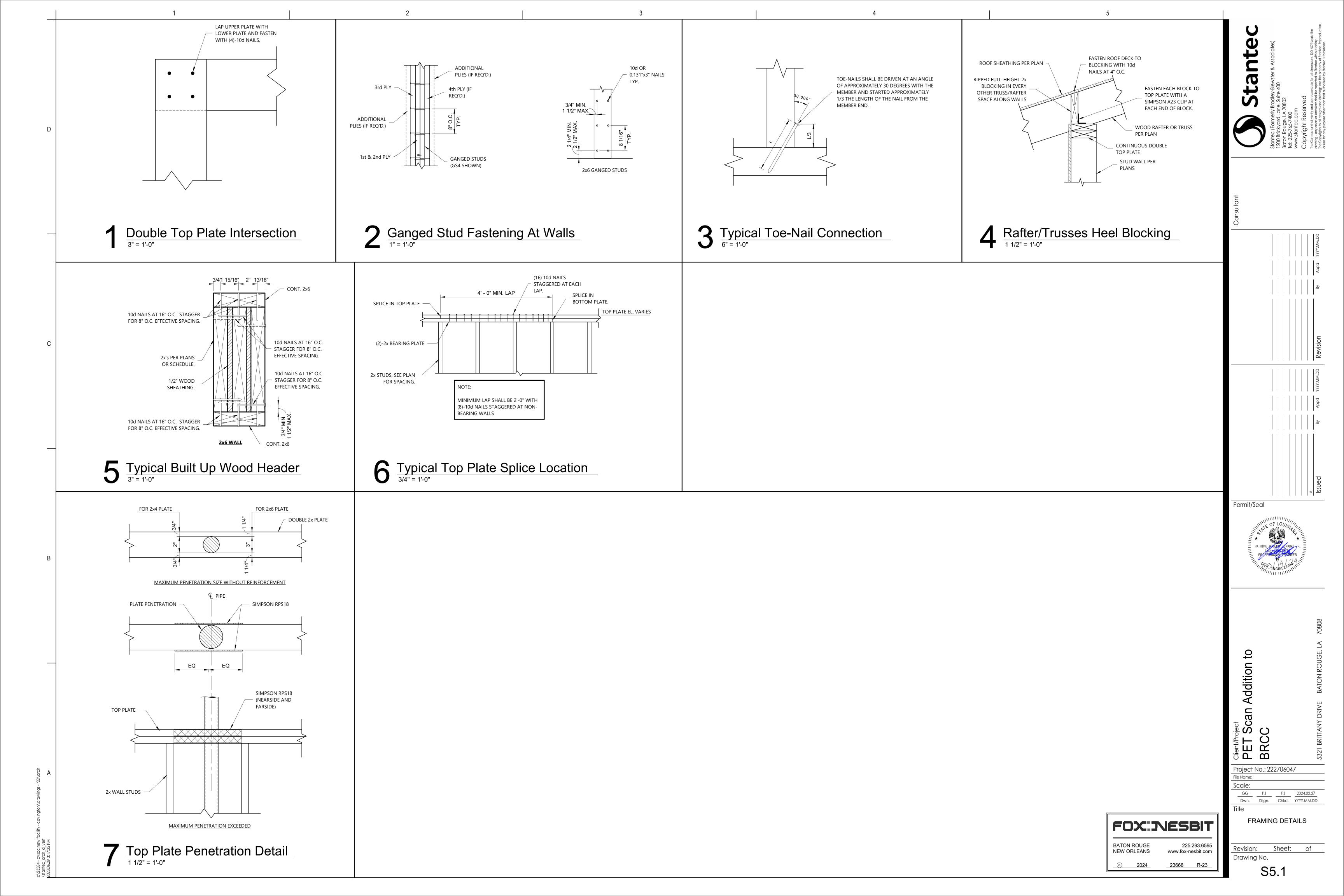
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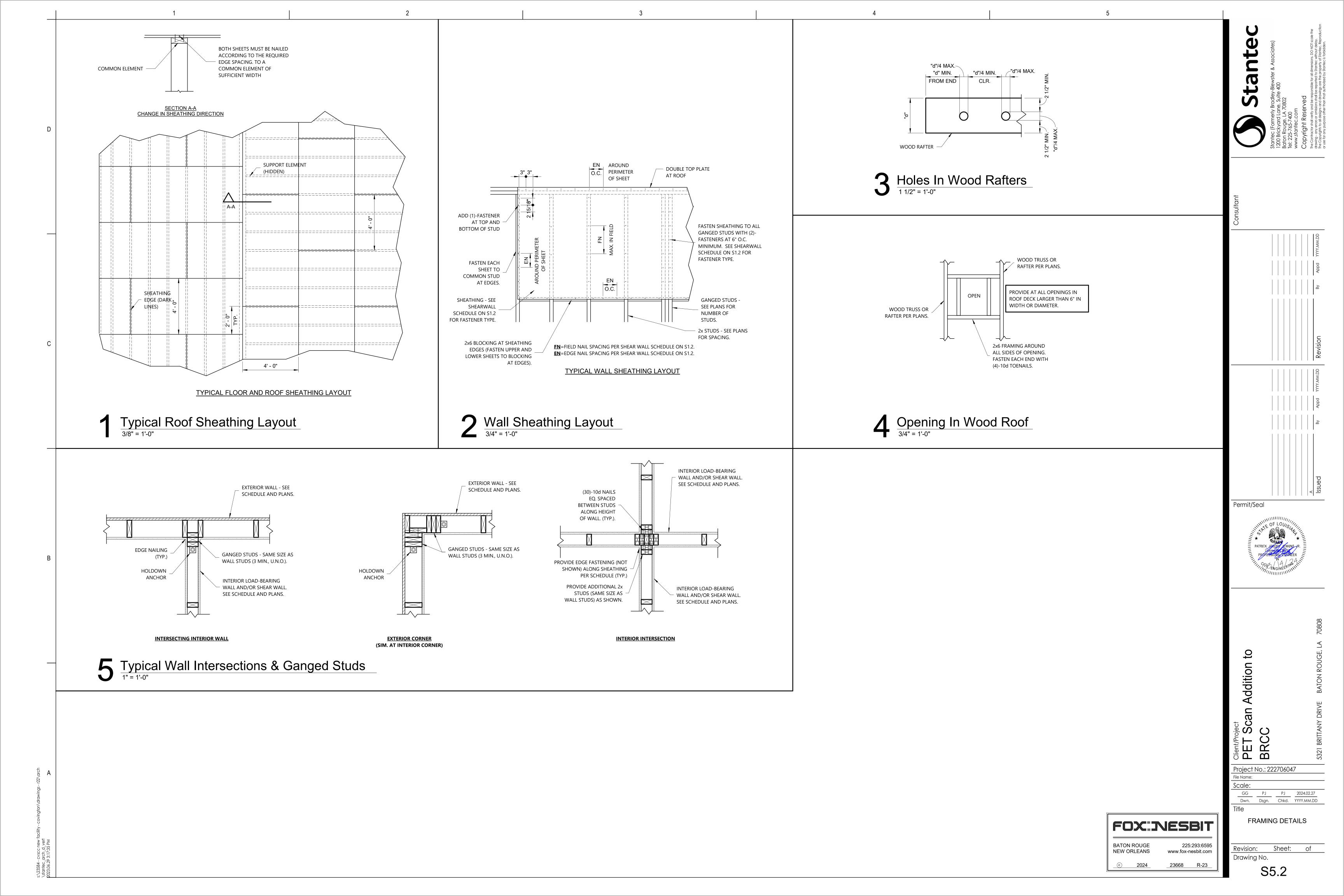
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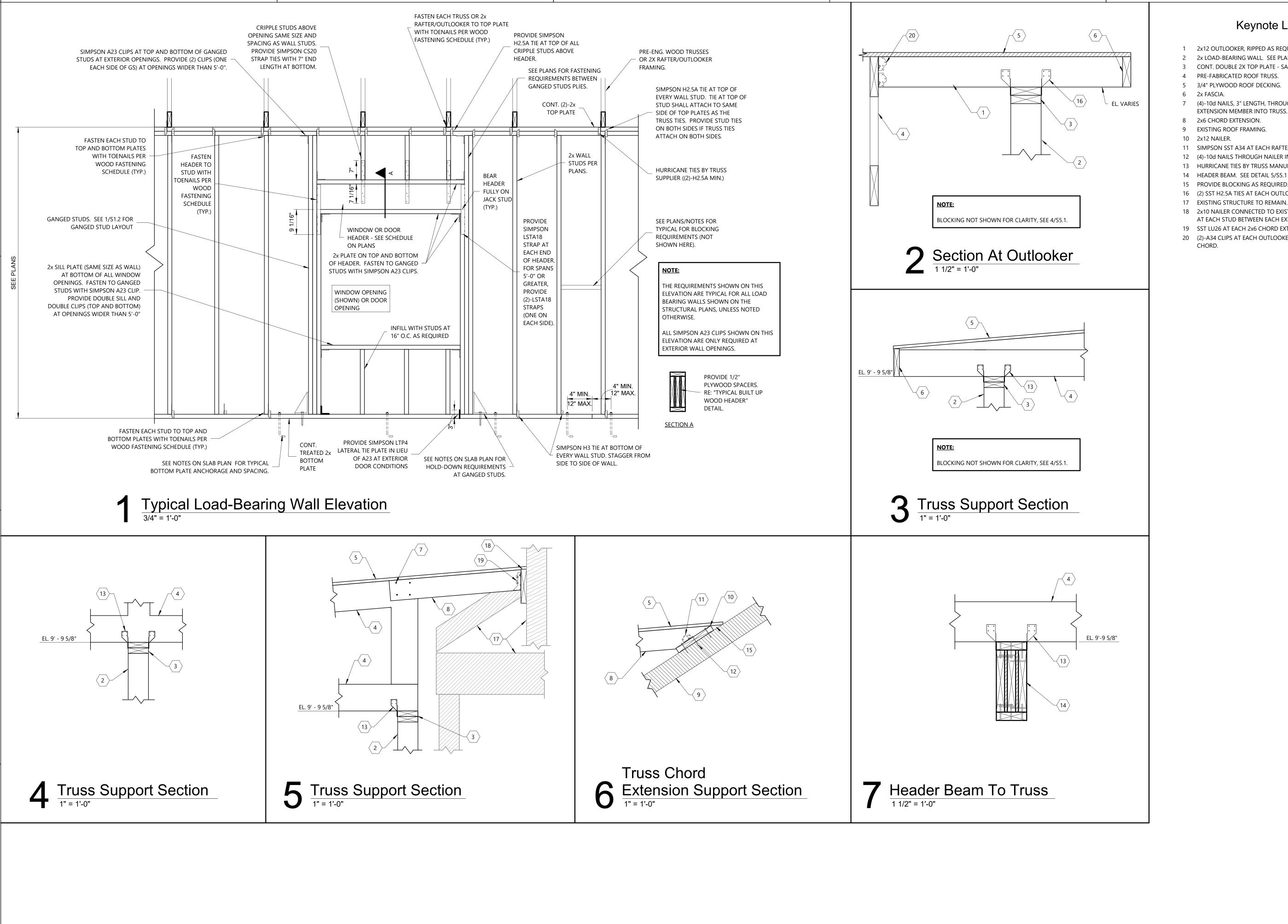


1 Building Section 2









Keynote Legend

1 2x12 OUTLOOKER, RIPPED AS REQUIRED.

2 2x LOAD-BEARING WALL. SEE PLANS FOR INFORMATION.

3 CONT. DOUBLE 2X TOP PLATE - SAME SIZE AS WALL.

4 PRE-FABRICATED ROOF TRUSS. 5 3/4" PLYWOOD ROOF DECKING.

6 2x FASCIA.

7 (4)-10d NAILS, 3" LENGTH, THROUGH EACH 2x6 CHORD EXTENSION MEMBER INTO TRUSS.

8 2x6 CHORD EXTENSION. 9 EXISTING ROOF FRAMING.

10 2x12 NAILER.

11 SIMPSON SST A34 AT EACH RAFTER INTO NAILER.

12 (4)-10d NAILS THROUGH NAILER INTO EACH EXISTING RAFTER. 13 HURRICANE TIES BY TRUSS MANUFACTURER. ((2)-H2.5A MIN.)

14 HEADER BEAM. SEE DETAIL 5/S5.1 FOR MORE INFORMATION.

15 PROVIDE BLOCKING AS REQUIRED.

16 (2) SST H2.5A TIES AT EACH OUTLOOKER.

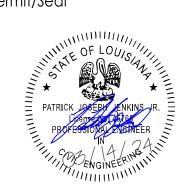
18 2x10 NAILER CONNECTED TO EXISTING STUDS WITH (4)-10d NAILS AT EACH STUD BETWEEN EACH EXISTING RAFTER TO REMAIN.

19 SST LU26 AT EACH 2x6 CHORD EXTENSION.

20 (2)-A34 CLIPS AT EACH OUTLOOKER CONNECTED TO TRUSS TOP CHORD.

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GENERAL NOTES

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#### A. APPLICABLE DESIGN CODES & MISCELLANEOUS

**INTERNATIONAL BUILDING CODE 2021** AMERICAN CONCRETE INSTITUTE 318 AMERICAN INSTITUTE OF STEEL CONSTRUCTION

**IBC CHAPTER 17 SPECIAL INSPECTIONS:** THE CONTRACTOR IS REQUIRED TO PROVIDE SPECIAL INSPECTIONS IN ACCORDANCE WITH CHAPTER 17 OF IBC 2021. THE GENERAL CONTRACTOR IS REQUIRED TO

ENGAGE AND ACCOMMODATE THE REQUIRED SPECIAL INSPECTIONS BY PROVIDING ACCESS TO ELEMENTS REQUIRED FOR INSPECTION AND BY NOTIFYING THE TESTING AGENCY 48 HOURS PRIOR TO A REQUIRED INSPECTION EVENT. THE CONTRACTOR SHALL PROVIDE REPORTS FROM THE TESTING AGENCY INDICATING COMPLIANCE WITH THE IBC REQUIREMENTS FOR:

- CONCRETE CONSTRUCTION (IBC 1705.3) - SOILS (IBC 1705.6) - WIND RESISTANCE (IBC 1705.11) (IN APPLICABLE WIND SPEEDS ONLY)

### **STRUCTURAL OBSERVATIONS:**

STRUCTURAL OBSERVATIONS SHALL BE CONDUCTED BY THE ENGINEER OF RECORD TO ASSURE GENERAL COMPLIANCE WITH THE CONTRACT DOCUMENTS. THESE OBSERVATIONS WILL NOT TAKE THE PLACE OF THE CODE REQUIRED SPECIAL INSPECTIONS LISTED ABOVE OR ANY OTHER INSPECTIONS REQUIRED BY THE LOCAL BUILDING OFFICIAL. NOTIFY ENGINEER OF RECORD AND ARCHITECT FOR STRUCTURAL OBSERVATION VIA EMAIL A MINIMUM OF 72 HOURS PRIOR TO ANY OF THE FOLLOWING EVENTS:

- ALL CONCRETE/GROUT POURS (WITH IDENTIFICATION OF SPECIFIC ELEMENTS TO BE - PLACEMENT OF INTERIOR SHEATHING OR INSULATION COVERING WOOD FRAMING OR COLD-FORMED METAL FRAMING.

- PLACEMENT OF ROOFING COVERING ROOF DECK.

FAILURE TO NOTIFY MAY REQUIRE REMOVAL OF COMPLETED WORK.

PROVIDE COMPREHENSIVE ELECTRONICALLY TRANSMITTED PHOTOS OF ANY REQUESTED WORK TO ENGINEER PRIOR TO ANY OF THE ABOVE EVENTS IN LIEU OF OBSERVATION IF DEEMED ACCEPTABLE BY ENGINEER.

#### **B. DESIGN LOADS AND REQUIREMENTS SECTION**

(1) FIRST FLOOR DESIGN LOA	ADS
LIVE LOAD	- 100 PSF (REDUCIBLE)
LIVE LOAD	- 2000 LB (CONCENTRATED
PARTITION LIVE LOAD	- 15 PSF (NON-REDUCIBLE)

#### (2) ROOF DESIGN LOADS LIVE LOAD ----20 PSF (REDUCIBLE) --300 LB (CONCENTRATED) LIVE LOAD ---

GROUND SNOW LOAD ----- 0 PSF

(3) LATERAL DESIGN - WIND ULTIMATE DESIGN WIND SPEED (Vult)----- 125 MPH NOMINAL DESIGN WIND SPEED (Vasd)----- 97 MPH EXPOSURE CATEGORY -----RISK CATEGORY ---

INTERNAL PRESSURE COEFFICIENT ----- +/-0.18

### MWFRS - DIRECTIONAL PROCEDURE (4) LATERAL DESIGN -SEISMIC

ASCE 7-16	
IMPORTANCE FACTOR	1.0
$S_s - \cdots -$	0.088g
S <sub>1</sub>	0.057g
SITE CLASS	D
$S_{ds} - \!$	0.094g
S <sub>d1</sub>	0.091g
SEISMIC DESIGN CATEGORY	В
C <sub>s</sub>	0.0313
DESIGN BASE SHEAR	0.0313*V
R	3
<b>EQUIVALENT LATERAL-FORCE ANALY</b>	SIS METH

STEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE.

## C. GEOTECHNICAL

NO GEOTECHNICAL INFORMATION WAS FURNISHED FOR THIS PROJECT. IT IS ASSUMED THAT THE SOIL CONDITIONS FOR THIS DESIGN ARE FIRM, STABLE, AND NON-EXPANSIVE AND ARE CAPABLE OF SAFELY SUPPORTING 1500#/SQ FT BEARING PRESSURES. BY USING THESE PLANS, THE OWNER HOLDS HARMLESS THE ENGINEER FROM ALL LIABILITY AND DAMAGES ARISING FROM UNSTABLE SOIL CONDITIONS OR FROM CONDITIONS WHERE FACTORS CAUSE SOIL CONDITIONS TO BECOME UNSTABLE AND ALLOW MOVEMENT WITHIN THE FOUNDATION AND STRUCTURAL SYSTEM. IF CONDITIONS ARE FOUND TO BE OTHER THAN HEREIN STATED, STOP OPERATIONS AND NOTIFY FOX-NESBIT FOR THE NECESSARY INVESTIGATION AND CORRECTIONS TO PLANS MADE.

NET ALLOWABLE SOIL BEARING PRESSURE ISOLATED SPREAD FOOTINGS ----- 1,500 PSF CONTINUOUS (WALL) FOOTINGS ----- 1,500 PSF MINIMUM BEARING DEPTH ----- 2'-0" BELOW FINAL GRADE

TESTING AGENCY SHALL INSPECT FOUNDATION SUBGRADE FOR ADEQUACY TO ACHIEVE THE DESIGN BEARING CAPACITY PRIOR TO DRY BOTTOM/FOOTING PLACEMENT. NO PRECIPITATION EVENT SHALL OCCUR IN TIME BETWEEN SUBGRADE APPROVAL AND DRY BOTTOM/FOOTING PLACEMENT.

#### D. CONCRETE AND GROUT

CONCRETE MIXING, HANDLING, PLACING, AND CURING SHALL BE IN ACCORDANCE WITH ACI 301.

SEE THE "CONCRETE MIX REQUIREMENTS" TABLE FOR DESCRIPTIONS AND REQUIREMENTS OF CONCRETE TYPES.

FLY ASH IS NOT PERMITTED IN ANY CONCRETE FOR THIS PROJECT.

SLAG IS NOT PERMITTED IN ANY CONCRETE FOR THIS PROJECT.

ALL FLOOR DRAINS, DROPS, CURBS, ETC. SHALL BE COORDINATED WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.

SEE PLUMBING DRAWINGS FOR LOCATIONS OF ALL FLOOR DRAINS. SLOPE GROUND FLOOR SLAB AND ELEVATED SLABS AT ALL FLOOR DRAINS AWAY FROM WALLS IN ROOM TO LOW POINT AT FLOOR DRAIN WHICH SHALL BE SET 1/2" BELOW FINISHED FLOOR OF SLAB, UNLESS NOTED OTHERWISE.

VERIFY ALL SLAB EDGE DIMENSIONS AT DOORS AND FULL-HEIGHT WINDOWS WITH ARCHITECTURAL DRAWINGS PRIOR TO SETTING OF GROUND FLOOR SLAB EDGE FORMS. AT LOCATIONS WHERE SLAB EDGE EXTENDS PAST OUTSIDE EDGE OF DOOR OR FULL-HEIGHT WINDOW, SLOPE SLAB DOWN 1/4" FROM OUTSIDE FACE OF DOOR WINDOW TO SLAB EDGE, UNLESS NOTED OTHERWISE.

#### E. CONCRETE REINFORCEMENT

ALL REBARS SHALL BE GRADE 60 (FY = 60,000 PSI MIN.)

VAPOR RETARDER AT GROUND FLOOR SLABS TO BE 15 MIL. WITH TAPED JOINTS. REFERENCE SPECIFICATIONS FOR CAST-IN-PLACE CONCRETE FOR ADDITIONAL INFORMATION.

PROVIDE (2)-#5 L BARS (a=36",b=36") ONE TOP AND ONE BOTTOM AT THE OUTSIDE FACE OF ALL GRADE BEAM CORNERS.

PROVIDE (4)-#5 L BARS (a=36",b=36") TWO TOP AND TWO BOTTOM AT ALL GRADE BEAM INTERSECTIONS.

ALL WELDED WIRE MESH SHALL HAVE 12" MIN. LAP BETWEEN SHEETS.

PLACE AND SECURE ALL EMBEDDED ITEMS INCLUDING REINFORCING DOWELS, ANCHOR BOLTS, FORM SAVER DOWELS AND EMBED PLATES PRIOR TO PLACING OF CONCRETE. DO NOT WET STICK ANY OF THESE ITEMS. UNLESS NOTED OTHERWISE HEREIN OR PERMITTED BY ENGINEER OF RECORD IN WRITING.

### F. POST-INSTALLED ANCHORS

IF SPECIFIC POST-INSTALLED ANCHOR IS NOT INDICATED ON DRAWINGS, THEN THE FOLLOWING POST-INSTALLED ANCHORS OR ADHESIVE SHALL BE USED FOR THIS PROJECT UNLESS EQUAL SUBSTITUTIONS ARE SUBMITTED AND APPROVED.

- **EPOXY ADHESIVE** SET-3G BY SIMPSON STRONG TIE
- HIT-RE 500v3 BY HILTI
- DEWALT PURE110+ DEWALT AC200+

HEAVY DUTY SCREW ANCHORS

 TITEN HD BY SIMPSON STRONG-TIE KH-EZ BY HILTI

DEWALT SCREW BOLT+

ALL POST-INSTALLED ANCHORS SHALL BE INSTALLED WITH STRICT ADHERENCE TO THE MANUFACTURER'S WRITTEN INSTALLATION INSTRUCTIONS.

FOR ALL POST INSTALLED ANCHOR APPLICATIONS, HOLES SHALL BE DRILLED WITH A HAMMER DRILL, U.N.O.

ALL DRILLED HOLES FOR ADHESIVE ANCHORS SHALL BE BRUSHED AND BLOWN CLEAN WITH COMPRESSED AIR AS SPECIFIED BY THE MANUFACTURER.

ALL ADHESIVE ANCHORS SHALL BE INSTALLED IN DRY CONCRETE, U.N.O.

DO NOT INSTALL POST-INSTALLED ANCHORS INTO NEW CONCRETE UNTIL DESIGN 28-DAY COMPRESSIVE STRENGTH HAS BEEN ACHIEVED AND IN NO CASE LESS THAN 7 DAYS.

ALL POST-INSTALLED ANCHORS AND ACCESSORIES EXPOSED TO WEATHER SHALL BE HOT-DIP GALVANIZED (OR HAVE APPROVED EQUAL CORROSION RESISTANCE).

#### **G. WOOD FRAMING MEMBERS**

ALL WOOD FRAMING MEMBERS SHALL BE NO. 2 SOUTHERN YELLOW PINE AND SHALL BE IN ACCORDANCE WITH MINIMUM DESIGN PROPERTIES PROVIDED IN THE NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION.

ALL ROOF MEMBERS SHALL BE ATTACHED TO SUPPORTING MEMBERS USING HURRICANE TIES.

PROVIDE 2x HORZ. BLOCKING (SAME SIZE AS WALL STUD) AT ALL EDGES OF SHEATHING (OSB OR GYP. BOARD) ON LOAD BEARING WALLS AND SHEAR WALLS SHOWN ON THESE PLANS.

BOTTOM PLATE ANCHORS SHALL BE LOCATED NO MORE THAN 12 INCHES AND NO LESS THAN 4 INCHES FROM ENDS OR PENETRATIONS OF BOTTOM PLATE. SEE PLANS FOR REQUIRED ANCHORS AND TYPICAL SPACING.

ALL PROPRIETARY WOOD CONNECTION HARDWARE SPECIFIED ON THESE PLANS SHALL BE INSTALLED PER THE MANUFACTURER'S REQUIREMENTS, INCLUDING PROPER TYPE AND QUANTITY OF FASTENERS.

SILL PLATES AND OTHER MEMBERS EXPOSED TO WEATHER OR IN CONTACT WITH CONCRETE OR MASONRY SHALL BE PRESSURE TREATED FOR MOISTURE RESISTANCE.

ALL CONNECTORS EXPOSED TO WEATHER OR IN CONTACT WITH TREATED WOOD SHALL BE FABRICATED WITH A MINIMUM G185 GALVANIZED COATING IN ACCORDANCE WITH ASTM A653 (I.E. SIMPSON ZMAX) OR HOT-DIP GALVANIZED IN ACCORDANCE WITH ASTM A653. ALL OTHER CONNECTORS SHALL BE FABRICATED WITH A MINIMUM G90 GALVANIZED COATING IN ACCORDANCE WITH ASTM A653.

WOOD FASTENERS (INCLUDING NAILS, BOLTS, NUTS, WASHERS, ETC.) SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH ASTM A153 AT ALL CONNECTIONS EXPOSED TO WEATHER, IN CONTACT WITH TREATED WOOD, AND AT ALL ROOF AND EXTERIOR WALL SHEATHING.

ALL LAG SCREWS WITH A DIAMETER OF 3/8" OR GREATER SHALL BE INSTALLED USING A LEAD HOLE WITH A DIAMETER EQUAL TO 60 TO 70 PERCENT OF THE SHANK DIAMETER. THE LEAD HOLE LENGTH SHALL BE EQUAL TO THE LAG SCREW EMBEDMENT.

ALL WOOD SHEATHING SHALL HAVE VISIBLE APA RATING STAMP.

#### **H. MEDICAL EQUIPMENT COORDINATION**

THE GENERAL CONTRACTOR IS RESPONSIBLE FOR ALL COORDINATION WITH MEDICAL EQUIPMENT SUPPLIERS TO VERIFY THAT THE STRUCTURAL DETAILS PROVIDED ON THESE DRAWINGS ARE ADEQUATE FOR INSTALLATION AND PERFORMANCE OF MEDICAL EQUIPMENT. ANY CONFLICTS OR INADEQUATE SUPPORT CONDITIONS SHALL BE REPORTED TO THE DESIGN TEAM PRIOR TO THE SUBMITTAL OF SHOP DRAWINGS OR ORDERING OF MATERIALS. CONFLICTS OR CONDITIONS NOT REPORTED WILL BE RESOLVED AT CONTRACTOR'S EXPENSE. THE CONTRACTOR WILL BE RESPONSIBLE FOR ANY REPAIRS, MODIFICATIONS, OR ADDITIONAL MATERIAL REQUIRED DUE TO IMPROPER OR UNTIMELY COORDINATION.

CONCRETE SLABS OR TOPPINGS WHICH SUPPORT MEDICAL EQUIPMENT SHALL MEET FLATNESS AND LEVELNESS REQUIREMENTS AS REQUIRED BY EQUIPMENT SUPPLIER. THE CONTRACTOR IS RESPONSIBLE FOR PROPERLY ANCHORING MEDICAL EQUIPMENT INTO GROUND FLOOR SLABS AS REQUIRED BY MANUFACTURER'S RECOMMENDATIONS. VERIFY THAT ALL SLAB DEPRESSIONS AND TRENCHES ARE PROPERLY LOCATED. CONTACT ENGINEER PRIOR TO SUBMITTAL OF REBAR SHOP DRAWINGS IF ADDITIONAL SLAB TRENCHES OR DEPRESSIONS ARE REQUIRED FOR MEDICAL EQUIPMENT.

### **I. RENOVATIONS**

**EXISTING CONDITIONS:** 

SUPPORT REMOVAL.

ALL DIMENSIONS AND CONDITIONS TYING INTO OR GOVERNED BY EXISTING CONSTRUCTION ARE APPROXIMATE AND ARE NOT PURPORTED TO BE EXACT. ALL SUCH DIMENSIONS AND CONDITIONS SHALL BE FIELD VERIFIED BY THE CONTRACTOR PRIOR TO THE PREPARATION OF SHOP DRAWINGS. FIRST SUBMITTAL OF SHOP DRAWINGS MUST CONTAIN CORRECT CONDITIONS AND DIMENSIONS OBTAINED FROM THE FIELD. IF CONDITIONS AND DIMENSIONS VARY GREATLY FROM THOSE SHOWN, THE CONTRACTOR SHALL NOTIFY THE ARCHITECT BEFORE PREPARATION OF SHOP DRAWINGS.

SHORE AND BRACE ALL EXISTING FRAMING AS REQUIRED IN ORDER TO ACCOMPLISH WORK SHOWN ON DRAWINGS. DESIGN OF ALL SHORING SHALL BE PROVIDED BY THE CONTRACTOR. SHORING SHALL BE PROVIDED AT OR

IMMEDIATELY ADJACENT TO LOCATION OF EXISTING

**DEMOLITION OF EXISTING CONSTRUCTION:** PRIOR TO THE START OF DEMOLITION OR EXPLORATORY WORK, THE OWNER SHALL EMPLOY AN INDEPENDENT TESTING LABORATORY TO SURVEY THE EXISTING SITE CONDITIONS FOR THE PRESENCE OF HAZARDOUS MATERIALS SUCH AS, BUT NOT LIMITED TO, LEAD-BASED PAINT, ASBESTOS, MOLD, ETC. IF THE TESTS RESULTS ARE POSITIVE FOR ANY HAZARDOUS MATERIALS, THE OWNER SHALL EMPLOY A REMEDIATION FIRM TO REMOVE THE HAZARDOUS MATERIALS IN COMPLIANCE WITH THE GUIDELINES AND REGULATIONS OF LOCAL, STATE, AND FEDERAL GOVERNMENTS BEFORE DEMOLITION OR

EXPLORATORY WORK MAY COMMENCE.

**DAMAGE TO EXISTING CONSTRUCTION:** ALL WORK SHALL BE DONE IN A MANNER WHICH WILL NOT DAMAGE ADJACENT EXISTING CONSTRUCTION WHICH IS TO REMAIN.

PATCHING MATERIALS AND INSTALLATION: ALL MATERIALS USED FOR PATCHING SHALL MATCH EXISTING MATERIALS IN APPEARANCE AND QUALITY. WORKMANSHIP SHALL BE IN CONFORMANCE WITH TODAY'S STANDARDS BUT SHALL BE NO LESS IN QUALITY THAN ANY OF THE ADJACENT WORKMANSHIP IN THE AREA BEING PATCHED.

PENETRATIONS IN EXISTING MASONRY/BRICK WALLS: ALL NEW PENETRATIONS THROUGH EXISTING MASONRY WALLS OR CONCRETE SLAB GREATER THAN 3" AND NOT SHOWN HEREIN THESE DRAWINGS SHALL BE APPROVED BY FOX-NESBIT IN WRITING.

PRICING/BIDDING: ALL ELEMENTS SHALL BE CONSIDERED NEW FOR PRICING/BIDDING UNLESS SPECIFICALLY IDENTIFIED AS EXISTING.

## J. NOTICE

THE USE OF REPRODUCTION OF THESE CONTRACT DRAWINGS BY THE CONTRACTOR, SUB-CONTRACTOR, ERECTOR, FABRICATOR, OR MATERIAL SUPPLIER IN LIEU OF PREPARED SHOP DRAWINGS SIGNIFIES HIS ACCEPTANCE OF ALL INFORMATION SHOWN HEREON AS CORRECT AND OBLIGATES HIMSELF TO ANY JOB EXPENSE, REAL OR IMPLIED, ARISING FROM ANY ERRORS THAT MAY BE PRESENT

IN THE EVENT OF CONFLICTING OR DIFFERING REQUIREMENTS INDICATED ON THE STRUCTURAL DRAWINGS AND/OR SPECIFICATIONS THAT HAVE NOT BEEN CLARIFIED OR CHANGED, THE CONTRACTOR SHALL PROVIDE THE BETTER QUALITY, GREATER QUANTITY, OR MORE STRINGENT UNLESS DIRECTED OTHERWISE BY ARCHITECT/ENGINEER.

THE CONTRACT STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION, EXCEPT WHERE SPECIFIC REQUIREMENTS ARE PROVIDED. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE STRUCTURE AND PERSONNEL DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE, BUT NOT BE LIMITED TO, BRACING, SHORING FOR LOADS DUE TO CONSTRUCTION EQUIPMENT, EXCAVATION PROTECTION, SCAFFOLDING, JOB SITE SAFETY, ETC. STRUCTURAL ENGINEER SHALL NOT BE RESPONSIBLE FOR THE CONTRACTOR'S MEANS, METHODS, TECHNIQUES, PROCEDURES, OR SEQUENCES OF CONSTRUCTION.

#### **K. FIELD VERIFICATIONS**

CONTRACTOR TO FIELD MEASURE ALL NEEDED DIMENSIONS PRIOR TO ORDERING MATERIAL.

CONTRACTOR IS RESPONSIBLE FOR FIELD VERIFICATION OF ALL DETAILS, GEOMETRY, DIMENSIONS, AND ELEVATIONS PRIOR TO ORDERING/FABRICATION OF MATERIALS. CONTACT ARCHITECT AND ENGINEER IMMEDIATELY IF ANY DIMENSIONS, DETAILS, OR ELEVATIONS ARE NOT FOUND TO MATCH THOSE SHOWN ON THE PLANS.

L. ABBREVIATION	<u>S</u>
@	AT
A/E	ARCHITECT/ENGINEER
A.F.F	ABOVE FINISHED FLOOR
ARCH	ARCHITECTURAL
BF	BRACED FRAME
BM	BEAM
B.O.C	BEAM ON COLUMN
B.O.S	BOTTOM OF STEEL
BOT	BOTTOM
BTM	BOTTOM
B/W	
BTWN	BETWEEN
C.F.M.F. OR CFMF -	COLD-FORMED METAL FRAMING
C.I.P	
	CENTER OF GRAVITY
	COMPLETE JOINT PENETRATION
C.L. OR CL	CENTER LINE
C.O.B	COLUMN ON BEAM
COL	COLUMN
CONT	CONTINUOUS
CONNX	
EL	
ELEV	
ELEC	
E.O.A	
	ENGINEER OF RECORD
E.O.S	
EXIST	
F.F	
FIN. FLR	
GA	
	GENERAL CONTRACTOR
	GLUE-LAMINATED
GR. BM	
	DETAIL APPLIES HIGH
	HEADED STUD ANCHOR
	HEADED STUD ANCHORS
	HOLLOW STRUCTURAL SECTION
	DETAIL APPLIES LOW
	METAL BUILDING SUPPLIER
MECH	
	MECHANICAL, ELECTRICAL, PLUMBIN
O.C	
	ON CENTER EACH WAY
OPP	
PEMBS	PRE-ENGINEERED METAL BUILDING

SUPPLIER

- SIMILAR

- TOP OF

WITH

- STRENGTH

- REINFORCEMENT

-- ROOF TOP UNIT

- TOP OF CONCRETE

--VERIFY ON JOBSITE

-- WELDED WIRE FABRIC

--- UNLESS NOTED OTHERWISE

- TOP OF JOIST

--TOP OF SLAB

- WIDE FLANGE

- POST TENSION OR POST-TENSIONED

-- POST TENSION OR POST-TENSIONED

-PLATE

RTU --

SIM. -

STR. ----

T.O.C. --

T.O.J. -

T.O.S. -

U.N.O. -

V.O.J. --

WF -----

WWF -----

T.O. -----

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CONCRETE MIX REQUIREMENTS							
USAGE	AGGREGATE	MIN. CEMENT (lb/yd³)	SLUMP (inches)	7 DAY STR. (psi)	28 DAY STR. (psi)	WATER REDUCER	REMARKS
GRADE BEAMS	1	489	4	2000	3000	В	
SLAB ON GRADE	1)	545	7	2700	4000	A	
DRY BOTTOMS					1500		
ALL OTHERS	1	545	7	2700	4000	A	

NOTES:

1 REGULAR SAND AND GRAVEL (145 pcf)

(A) MID-RANGE WATER REDUCER

USED, THEN SLUMP SHALL BE 7".

(B) CONTRACTOR'S OPTION - IF WATER REDUCER IS

THE SLUMP IN THE TABLE ABOVE IS GIVEN AT POINT OF PLACEMENT. THE ALLOWABLE TOLERANCE FOR SLUMP IS PLUS OR MINUS ONE INCH FROM THE VALUES GIVEN IN THE TABLE.

CONCRETE NOT MEETING THE SPECIFIED SEVEN DAY STRENGTH SHALL EITHER BE REMOVED OR CONSTRUCTION MUST BE STOPPED IN THE QUESTIONABLE AREA UNTIL THE 28 DAY TEST VALUES HAVE BEEN APPROVED.

SEE GENERAL NOTES FOR ADDITIONAL REQUIREMENTS.

REFERENCE SPECIFICATION SECTION 03 3000- FOR PROPORTIONING AND DESIGN OF MIXES.

# REBAR LAP SPLICE REQUIREMENTS (MIN.)

LOCATION	BEAMS AND	BEAMS AND FOUNDATIONS WALLS AND SLABS			
f'c BAR	3000 PSI	4000 PSI	3000 PSI	4000 PSI	
#3	22"	19"	16"	16"	
#4	29"	25"	17"	16"	
#5	36"	31"	26"	22"	
#6	36"	36"	36"	36"	
#7	42"	42"	42"	42"	
#8	42"	42"	42"	42"	

#### **GENERAL NOTES:**

LAP SPLICE LENGTHS ABOVE APPLY TO ALL REINFORCING BARS FOR THIS PROJECT, UNLESS SPECIFICALLY NOTED OTHERWISE IN THESE PLANS.

ALL LAP SPLICES PROVIDED ABOVE ARE FOR NORMAL WEIGHT CONCRETE AND GRADE 60 REINFORCING BARS IN TENSION. SPLICES FOR WALL AND SLAB BARS ARE BASED ON A MINIMUM OF 1" CLEAR COVER.

STAND	ARD BAR I	BEND DIAC	SRAMS
"a" DIM  WIO "q"	"a" DIM	"a" DIM	"a" DIM  "c" DIM
<u>L BAR</u>	<u>U BAR</u>	TIES AND HOOPS	<u>Z BAR</u>
NOTES:			("d" = 90 DEGREES IF NOT PROVIDED)
SEE DETAILS AND KEYN	NOTES FOR DIMENSIONS OF	ALL BARS AND TIES.	
	MENSIONS ARE NOT GIVEN IONS FROM OUTER EDGE O		
	RWISE, ALL BAR BEND DIAME ATEST VERSION OF ACI 318.	ETERS ("D") SHALL BE IN	

# GENERAL WOOD SHEATHING/DECKING FASTENER REQUIREMENTS

			•	
TYPE	SHEATHING/DECKING TYPE	GENERAL FASTENER SPACING	EDGE FASTENER SPACING	REMARKS
ROOF	3/4" EXTERIOR GRADE APA RATED EXPOSURE 1 PLYWOOD SHEATHING	12" O.C.	6" O.C.	A B

A LL FASTENERS SHALL BE 10d COMMON NAILS.

ATTACH SHEATHING/DECKING TO SUPPORTS AT 6" O.C. AROUND THE EDGES OF ALL SHEETS, AND AT 12" O.C. IN THE FIELDS OF THE SHEETS.

B ATTACH ROOF DECK TO ALL SUPPORTS AT ROOF OVERHANGS AND AT 8'-0" WIDE STRIPS AROUND PERIMETER OF ENCLOSED BUILDING SPACES. AT 6" O.C. (MAX.)

	COMPONENTS & CLADDING DESIGN WIND PRESSURES (PSF)															
ZONE		ROOF											OVERHANG WALLS			
EWA (SF)		1		2	;	3	2	2'	(3)	3'	1&2	3	4	1	ļ	5
≤10	16	-37.0	16	-42.8	16	-57.2	16	-51.4	16	-80.3	-82.6	-111.5	31.2	-33.8	31.2	-41.6
20	16	-37.0	16	-41.9	16	-52.0	16	-50.6	16	-71.6	-80.4	-101.4	29.8	-32.4	29.8	-38.9
50	16	-37.0	16	-40.8	16	-45.1	16	-49.4	16	-60.1	-77.4	-88.1	28.0	-30.6	28.0	-35.2
100	16	-37.0	16	-39.9	16	-39.9	16	-48.6	16	-51.4	-75.2	-78.0	26.6	-29.2	26.6	-32.4
200	16	-37.0	16	-39.9	16	-39.9	16	-48.6	16	-51.4	-73.8	-76.6	25.2	-27.8	25.2	-29.7
≥500	16	-37.0	16	-39.9	16	-39.9	16	-48.6	16	-51.4	-72.0	-74.8	23.4	-26.0	23.4	-26.0

- 1. EWA IS EFFECTIVE WIND AREA OF A STRUCTURAL COMPONENT.
- 2. FOR ZONE DEFINITIONS, SEE ASCE 7-16 FIGURES 30.4 (AS APPLICABLE). 3. PLUS AND MINUS SIGNS SIGNIFY PRESSURES ACTING TOWARD AND AWAY FROM THE SURFACE RESPECTIVELY.
- 4. EDGE WIDTH DIMENSION "a"=3.0 FT.
- 5. LINEAR INTERPOLATION MAY BE USED BETWEEN EWA VALUES PROVIDED IN THE ABOVE TABLE.
- 6. PRESSURES IN THE ABOVE TABLE ARE ULTIMATE LEVEL PRESSURES BASED ON LATERAL DESIGN WIND PARAMETERS IN GENERAL NOTES.

WOOD FASTENING SCHEDULE						
CONNECTION	FASTENER TYPE AND NO.	LOCATION				
JOIST TO SILL, GIRDER, OR TOP PLATE	3-10d COMMON	TOENAIL				
BRIDGING TO JOIST OR TRUSS	2-10d COMMON	TOENAIL EA. END				
BOTTOM PLATE TO JOIST, BLOCKING, OR FRAMING BELOW	16d COMMON at 6" o.c.	TYP. FACE NAIL				
TOP PLATE TO STUD	4-10d COMMON	TOENAIL				
STUD TO BOTTOM PLATE	4-10d COMMON	TOENAIL				
DOUBLE STUD	10d COMMON at 8" o.c.	FACE NAIL				
DOUBLE TOP PLATES	10d COMMON at 6" o.c.	TYP. FACE NAIL				
DOUBLE TOP PLATES - LAP SPLICES	16-10d COMMON	LAP SPLICE				
BLOCKING BTWN. JOISTS OR FRAMING MEMBERS	3-10d COMMON	TOENAIL				
TOP PLATE INTERSECTIONS	4-10d COMMON	FACE NAIL				
TRUSS, OUTLOOKER, OR JOIST TO EXTERIOR WALL NAILER	4-10d COMMON	TOENAIL				
TRUSS TO INTERIOR WALL TOP PLATES OR NAILER	2-10d COMMON	TOENAIL				
CONTINUOUS HEADER TO STUD	4-10d COMMON	TOENAIL				
BUILT-UP CORNER STUDS	10d COMMON at 6" o.c.	FACE NAIL				
2x fascia to roof truss, Joist, or outlooker	2-10d COMMON	FACE NAIL				

USE THE REQUIRED FASTENERS UNLESS NOTED

OTHERWISE ON PLANS.

 $8d = 2 \frac{1}{2}$ " LONG x 0.131" DIA. 10d = 3" LONG x 0.148" DIA.  $16d = 3 \frac{1}{2}$ " LONG x 0.162" DIA.

**COMMON NAIL SIZES:** 

Addition S S S S S

Permit/Seal

 GG
 PJ
 PJ
 2024.02.27

 Dwn.
 Dsgn.
 Chkd.
 YYYY.MM.DD

SCHEDULES

Project No.: 222706047

Revision: Sheet: of Drawing No.

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