GENERAL NOTES

GENERAL CONDITIONS AND COORDINATION

1. NOTES SHOWN ON GENERAL NOTES SHEET SHALL GOVERN THE MINIMUM STANDARDS FOR MATERIALS, WORKMANSHIP, AND GENERAL CONSTRUCTION PRACTICES UNLESS NOTED OTHERWISE IN SPECIFICATIONS OR ON DRAWINGS.
2. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO OBTAIN AND DISTRIBUTE ALL CURRENT CONTRACT DOCUMENTS AND ADDENDA TO SUPPLIERS AND SUB-CONTRACTORS FOR THE USE OF SHOP DRAWING PRODUCTION AND FABRICATION PRIOR TO CONSTRUCTION.
3. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO COMPARE THE ARCHITECTURAL, STRUCTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND OTHER DRAWINGS, AND REPORT ANY DISCREPANCIES AMONG OR WITHIN THE DRAWING SETS PRIOR TO FABRICATION OR CONSTRUCTION.
4. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY ALL DIMENSIONS, FLOOR ELEVATIONS, DROPS, SLOPES, DRAINS, EMBEDDED ITEMS, ETC., PRIOR TO CONSTRUCTION.
5. THE DETAILS AND SECTIONS SHOWN ON STRUCTURAL DRAWINGS APPLY GENERALLY TO ALL AREAS OF SIMILAR OR LIKE CONDITIONS THROUGHOUT THE DRAWINGS.
6. STRUCTURAL DRAWINGS INDICATE TYPICAL AND INDIVIDUAL SPECIFIC CONDITIONS ONLY. IT IS THE RESPONSIBILITY OF THE CONTRACTOR/SUB-CONTRACTOR TO PREPARE SHOP DRAWINGS DETAILING CONDITIONS IN ACCORDANCE WITH SPECIFIED STANDARDS AND SPECIFIC REQUIREMENTS OF THIS PROJECT AS INDICATED ON DRAWINGS.
7. THE USE OF THESE STRUCTURAL DRAWINGS BY ANY CONTRACTOR, SUB-CONTRACTOR, MATERIAL SUPPLIER, FABRICATOR, OR ERECTOR WITHOUT THE PREPARATION OF SHOP DRAWINGS REPRESENTS HIS ACCEPTANCE OF THESE DRAWINGS AS COMPLETE AND CORRECT. AS A RESULT, ANY EXPENSE ACQUIRED AS A RESULT OF ERRORS OCCURRING ON DRAWINGS IS THE RESPONSIBILITY OF THE INDIVIDUAL PARTY.
8. SHOP DRAWINGS MAY BE SUBMITTED TO ENGINEER FOR REVIEW FOR CORRECTNESS OF STRUCTURAL INTENT. CONTRACTOR, SUB-CONTRACTOR, MATERIAL SUPPLIER, FABRICATOR, OR ERECTOR SHOULD ANTICIPATE A MINIMUM 10 BUSINESS DAY REVIEW PERIOD BY ENGINEER.
9. THE DESIGN AND PROVISION FOR ALL TEMPORARY SUPPORTS OR FRAMING, AND NON-STRUCTURAL FRAMING IS THE RESPONSIBILITY OF THE CONTRACTOR. TEMPORARY SUPPORTS SHALL NOT OVERSTRESS OR CAUSE DAMAGE TO THE PERMANENT STRUCTURAL ELEMENTS. REFERENCE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR ALL NON-STRUCTURAL FRAMING REQUIRED.
10. THE STRUCTURAL DRAWINGS AND ITEMS SHOWN HEREIN REPRESENT THE FINISHED STRUCTURE AND DO NOT NECESSARILY REPRESENT THE MEANS OR METHODS OF CONSTRUCTION. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR SUPERVISING THE WORK, AND THE MEANS, METHODS, PROCEDURES, TECHNIQUES, AND SEQUENCES OF CONSTRUCTION.
11. THE STRUCTURE SHOWN HEREIN IS STRUCTURALLY SOUND WHEN ALL HORIZONTAL AND LATERAL PERMANENT BRACING INDICATED ON DRAWINGS IS INSTALLED IN THEIR ENTIRETY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL TEMPORARY SUPPORT OF ALL ELEMENTS TO RESIST GRAVITY, EARTH, WIND, SEISMIC, AND CONSTRUCTION LOADS DURING CONSTRUCTION.
12. ALL ELEVATIONS SHOWN ARE FOR STRUCTURAL REFERENCE PURPOSES ONLY. REFER TO CIVIL FOR DATUM ELEVATIONS.

DESIGN CODES/STANDARDS

1. GOVERNING BUILDING CODE: 2012 INTERNATIONAL BUILDING CODE
2. DESIGN LOADS: MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES, ASCE 7-10
3. CONCRETE: BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE, AMERICAN CONCRETE INSTITUTE, ACI 318-11
4. POST TENSION FOUNDATION: POST TENSIONING INSTITUTE, STANDARD REQUIREMENTS FOR DESIGN AND ANALYSIS OF SHALLOW POST-TENSIONED CONCRETE FOUNDATIONS ON EXPANSIVE SOILS, PTI DC10.5-12
5. CONCRETE MASONRY: BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES, AMERICAN CONCRETE INSTITUTE, ACI 530-11
6. STRUCTURAL STEEL: SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS AND STEEL CONSTRUCTION MANUAL, AMERICAN INSTITUTE OF STEEL CONSTRUCTION, AISC 360-10 AND AISC 325-11
7. WOOD: NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION, AMERICAN FOREST & PAPER ASSOCIATION, NDS-12 W/ JUNE 2013 AWC ADDENDUM

LOADS AND DESIGN CRITERIA

1. DEAD LOADS

A.	FLOOR	45 PSF
	TRUSS TOP CHORD	40 PSF
	TRUSS BTM CHORD	5 PSF
B.	ROOF	15 PSF
	TRUSS TOP CHORD	10 PSF
	TRUSS BTM CHORD	10 PSF
	TRUSS BTM CHORD AT MECH ZONE	20 PSF
C.	STAIRS	20 PSF
D.	STAIR LANDINGS	20 PSF
	TRUSS TOP CHORD	15 PSF
	TRUSS BTM CHORD	5 PSF

2. LIVE LOADS

A.	FLOOR	40 PSF
B.	PUBLIC SPACES	100 PSF
C.	CORRIDORS	100 PSF
D.	ROOF	20 PSF
E.	ROOF MECH ZONE	50 PSF
F.	ROOF TRUSS BTM CHORD	10 PSF (NON-CONCURRENT) 40 PSF @ CATWALK
G.	STAIRS & LANDINGS	100 PSF
H.	STORAGE	125 PSF
J.	MECHANICAL	150 PSF

3. SNOW LOADS

A.	IMPORTANCE FACTOR	1.00
B.	GROUND SNOW LOAD	5 PSF

4. WIND LOADS

A.	RISK CATEGORY	III	
B.	BASIC WIND SPEED	135 MPH	
C.	EXPOSURE CATEGORY	B	
D.	C&C PRESSURES	REF S1.2 FOR TYP CONFIGURATIONS	
	EDGE DISTANCE 'EZ'	13 FT	
	ROOF EFFECTIVE AREA	10 SF (0.6W)	100 SF (0.6+0.6D)
	ROOF ZONE 1	18.2 PSF	7.5 PSF
	ROOF ZONE 2	31.8 PSF	14.2 PSF
	ROOF ZONE 2 OH	37.0 PSF	28.0 PSF
	ROOF ZONE 3	46.8 PSF	27.7 PSF
	ROOF ZONE 3 OH	62.2 PSF	33.1 PSF
	WALL EFFECTIVE AREA	10 SF (0.6W)	28 SF (0.6W)
	WALL ZONE 4	21.5 PSF	20.2 PSF
	WALL ZONE 5	26.6 PSF	24.0 PSF

5. SEISMIC LOADS

A.	STRUCTURAL SYSTEM		LIGHT FRAME WALLS W/ SHEAR PANELS (WOOD & OTHER MATERIALS)
B.	ANALYSIS PROCEDURE		EQUIVALENT LATERAL FORCE
C.	IMPORTANCE FACTOR		1.25
D.	SITE CLASS		D
E.	SEISMIC DESIGN CATEGORY		B
F.	MAPPED SRA	Ss	0.106 g
		S1	0.056 g
G.	DESIGN SRA	Sds	0.114 g
		Sd1	0.089 g

6. FOUNDATION DESIGN CRITERIA

A.	ALLOWABLE BEARING	2,000 PSF @ MIN 18" BELOW FIN GRADE

SOIL AND SUBSURFACE CONDITIONS

1. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO THOROUGHLY READ, UNDERSTAND THE DESIGN CRITERIA AND FOLLOW THE RELATED BUILDING PAD PREPARATION REQUIREMENTS SET FORTH IN THE GEOTECHNICAL REPORT PREPARED FOR THIS PROJECT.
2. FOUNDATION DESIGN IS BASED ON GEOTECHNICAL REPORT, PROJECT # EH1551185, PREPARED BY TERRACON, DATED 10/27/15.
3. BUILDING PAD PREPARATION SHALL BE IN ACCORDANCE WITH RECOMMENDATIONS IN GEOTECHNICAL REPORT. UPON COMPLETION OF PROOF-ROLLING, THE MOISTURE CONTENT OF THE SUBGRADE SHALL BE EVALUATED TO DEPTH OF 24". IF DRY CONDITIONS PERSIST, UNDERCUT TO A DEPTH OF 12" AND SCARIFY, WET AND RECOMPACT EXPOSED SUBGRADE IN ACCORDANCE WITH GEOTECHNICAL REPORT. ALL FOOTINGS AND GRADE BEAMS SHALL BEAR IN STRATUM 2 MEDIUM STIFF TO STIFF TAN AND GREY LEAN CLAY.
4. ANY FILL WORK WITHIN 10 FT OF BUILDING EXTENTS SHALL BE PROPERLY PLACED AND COMPACTED TO 95% OF MAXIMUM DRY DENSITY AS DEFINED IN ASTM D698 STANDARD PROCTOR TEST.
5. POSITIVE DRAINAGE SHALL BE PROVIDED AND MAINTAINED AWAY FROM THE BUILDING DURING CONSTRUCTION AND PERMANENTLY. STORED EXCAVATION MATERIAL AND/OR CONSTRUCTION MATERIALS SHALL NOT DISRUPT POSITIVE DRAINAGE AWAY FROM BUILDING.
6. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO PROVIDE ANY REQUIRED BACK FILLING OF WALLS, PIERS, FOOTINGS, ETC., SUCH THAT SYMMETRICAL LOADING OCCURS. IN THE EVENT THAT CONDITIONS PREVENT SUCH SYMMETRICAL LOADING, TEMPORARY SHORING SHALL BE PROVIDED AND MAINTAINED UNTIL PERMANENT HORIZONTAL AND VERTICAL BRACING ELEMENTS ARE PLACED AND PROPERLY SET.
7. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO MAINTAIN STABILITY OF EXCAVATIONS UNTIL PROPERLY BACK FILLED. EXCAVATIONS SHALL REMAIN FREE OF LOOSE DEBRIS/MATERIAL, AND WATER. EXCAVATIONS SHALL BE DE-WATERED AND ALL WET MATERIAL REMOVED/REPLACED PRIOR TO CONCRETE PLACEMENT.
8. HEAVY EQUIPMENT NECESSARY FOR SPREADING AND COMPACTING BACK FILL MATERIAL SHALL NOT BE OPERATED CLOSER THAN A DISTANCE EQUAL TO THE HEIGHT OF BACK FILL MATERIAL ABOVE THE WALL, PIER, FOOTING, ETC. HAND TAMPING SHALL BE USED TO COMPACT THE REMAINING AREA.
9. EXCAVATED MATERIAL MAY BE USED AS BACKFILL IF FOUND TO BE ACCEPTABLE TO THE GEOTECHNICAL ENGINEER. OTHERWISE, PROVIDE SELECT FILL IN ACCORDANCE WITH GEOTECHNICAL REPORT AS BACKFILL MATERIAL.
10. BUILDING PAD PREPARATION SHALL BE SUCH THAT THE THICKNESS OF FOUNDATION SLAB-ON-GRADE SHALL NOT BE REDUCED BY MORE THAN 5 PERCENT OF DEPTH SHOWN ON DRAWINGS.

SLAB-ON-GRADE FOUNDATION

1. LOCATION OF TREES IN CLOSE PROXIMITY CAN EFFECT LONG-TERM PERFORMANCE OF THE FOUNDATION. TREES TO BE REMOVED SHALL BE REMOVED PRIOR TO CONSTRUCTION OF THE FOUNDATION. CONTRACTOR SHALL CONSULT WITH APPROPRIATE JURISDICTIONAL OFFICIALS PRIOR TO REMOVAL OF PROTECTED TREES.
2. FINAL GRADE SHALL BE MAINTAINED TO PROVIDE POSITIVE DRAINAGE AWAY FROM THE FOUNDATION. FOUNDATION EXPOSURE AND SLOPE AWAY FROM FOUNDATION SHALL CONFORM WITH APPLICABLE CODE PROVISIONS. CONTRACTOR SHALL REFERENCE GRADING PLAN FOR FINISHED GRADE ELEVATIONS.
3. CONTRACTOR SHALL PROVIDE A 10 MIL POLY VAPOR BARRIER BENEATH ALL SLAB AREAS. BARRIER SHALL EXTEND A MINIMUM OF 12" DOWN BEAMS AND SHALL BE CUT OUT OF BOTTOM OF BEAM EXCAVATIONS TO FACILITATE FOUNDATION INSPECTIONS. CONTRACTOR SHALL PROVIDE A DOUBLE LAYER OF VAPOR BARRIER UNDER ALL CONSTRUCTION JOINTS. EXTENDING MIN 18" EACH SIDE OF THE JOINT. VAPOR BARRIER SHALL BE INSTALLED, LAPPED, AND TAPED IN ACCORDANCE WITH MANUFACTURERS RECOMMENDATIONS OR MIN 6".
4. WHERE SLAB BLOCK-OUTS ENCROACH INTO GRADE BEAMS, BEAM WIDTH SHALL BE INCREASED, TO MAINTAIN SPECIFIED MIN WIDTH EXCLUSIVE OF THE BLOCK-OUT, FOR THE FULL DEPTH OF THE BEAM. THE INCREASED BEAM WIDTH SHALL BE MAINTAINED AT MIN 30" EACH SIDE OF BLOCK-OUT. CONVENTIONAL REINFORCEMENT SHALL BE CONTINUOUS AROUND BLOCK-OUT.

CAST IN PLACE CONCRETE

1. CONCRETE WORK SHALL CONFORM TO THE FOLLOWING:
   A. ACI 318 - REINFORCED CONCRETE
   B. ACI 318.1 - PLAIN CONCRETE
   C. ACI 306R - COLD WEATHER CONCRETING
   D. ACI 305R - HOT WEATHER CONCRETING
   E. ACI 117 - STANDARD SPECIFICATION FOR TOLERANCES
2. CONCRETE USED FOR STRUCTURAL APPLICATIONS AS SHOWN ON DRAWINGS SHALL BE STANDARD WEIGHT, WITH 28-DAY COMPRESSIVE STRENGTH AS NOTED BELOW. COMPRESSIVE STRENGTH TESTING SHALL BE IN ACCORDANCE WITH ASTM C39 "STANDARD TEST METHOD FOR COMPRESSIVE STRENGTH OF CYLINDRICAL CONCRETE SPECIMENS."
3. CONCRETE SHALL HAVE A MAXIMUM SLUMP AS NOTED BELOW AND SLUMP SHALL BE DETERMINED IN ACCORDANCE WITH ASTM C143 "SLUMP OF PORTLAND CEMENT CONCRETE."
4. AGGREGATES USED FOR NORMAL WEIGHT CONCRETE SHALL HAVE A NOMINAL MAXIMUM COARSE AGGREGATE SIZE AS NOTED BELOW AND SHALL CONFORM TO ASTM C33 "SPECIFICATIONS FOR CONCRETE AGGREGATE."
5. CONCRETE SHALL BE PROPORTIONED TO MEET THE REQUIREMENTS OF ACI 318 CHAPTER 4. CONCRETE SHALL BE DESIGNED FOR EXPOSURE CLASS F0, S0, P0 AND C0 UNO.
6. CONCRETE MIX DESIGNS SHALL BE IN ACCORDANCE WITH THE REQS BELOW:

LOCATION	MIN F'c	SLUMP	MAXIMUM AGGREGATE SIZE	EXPOSURE CLASS
BEAMS	3000 PSI	4" +/- 1"	1 1/2"	C1
4" MIN SLAB ON GRADE	3000 PSI	4" +/- 1"	1 1/4"	C1

7. FLY ASH CONTENT SHALL BE MAX 25% OF CEMENT REPLACEMENT.
8. AIR ENTRAINMENT OF 1 1/2 PERCENT +/- 1 1/2 PERCENT, MAY BE USED AT THE CONTRACTORS OPTION. AIR ENTRAINMENT SHALL CONFORM TO ASTM C260 "AIR ENTRAINING ADMIXTURES FOR CONCRETE."
9. CONCRETE TESTING SHALL BE PROVIDED BY AN APPROVED AGENCY, AND IN ACCORDANCE WITH ASTM C31 "MAKING AND CURING CONCRETE TEST SPECIMENS IN THE FIELD."
10. CURING COMPOUNDS AND SURFACE HARDENERS SHALL BE APPROVED BY ENGINEER PRIOR TO USE. APPLICATION OF CURING COMPOUNDS AND SURFACE HARDENERS SHALL BE IN COMPLIANCE WITH MANUFACTURERS RECOMMENDATIONS.
11. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH SHALL BE PROTECTED BY WATERPROOFING AS DETAILED BY ARCHITECTURAL DRAWINGS.
12. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO COORDINATE FOUNDATION BLOCKOUTS AND EMBEDDED ITEMS NECESSARY FOR ARCHITECTURAL, MEP, CIVIL, ETC.
13. THE CONTRACTOR SHALL PROVIDE A SUBMITTAL OF EMBEDDED CONDUITS, PIPES, AND SLEEVES WHICH ARE BEYOND THE SCOPE DETAILED IN THE STRUCTURAL DRAWINGS.
14. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO PLACE AND FINISH CONCRETE SLABS WITH A MINIMUM FLATNESS OF FF = 35 AND A MINIMUM LEVELNESS OF FL = 25. ANY DEVIATION FROM THIS TOLERANCE THAT REQUIRES CUTTING OR ADDITIONAL FINISHING IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
15. HORIZONTAL CONSTRUCTION JOINTS ARE NOT PERMITTED UNLESS SPECIFICALLY SHOWN AND DETAILED ON STRUCTURAL PLANS. VERTICAL CONSTRUCTION JOINT LOCATIONS, OTHER THAN THOSE SHOWN ON PLAN, SHALL BE SUBMITTED TO ARCHITECT/ENGINEER FOR REVIEW. ADDITIONAL DETAILING AND REINFORCING MAY BE REQUIRED AND SPECIFIED BY THE ENGINEER FOR UNSCHEDULED CONSTRUCTION JOINTS, AND IS THE RESPONSIBILITY OF THE CONTRACTOR.
16. WHERE WIDTH AND DEPTH OF GRADE BEAM VARIES AT INTERSECTIONS, EXTEND THE LARGER OF THE BEAMS 3'-0" MIN BEYOND INTERSECTION AND SLOPE REINFORCEMENT OF LARGER BEAM ALONG LAP LENGTH OF SMALLER BEAM.

CONCRETE REINFORCING

1. REINFORCING STEEL SHALL BE GRADE 60, DOMESTIC, DEFORMED NEW BILLET STEEL BARS IN ACCORDANCE WITH ASTM A615.
2. REINFORCING STEEL DETAILING SHALL BE IN ACCORDANCE WITH THE AMERICAN CONCRETE INSTITUTE DETAILING MANUAL. ALL HOOKS AND BENDS IN REINFORCING STEEL SHALL CONFORM TO ACI DETAILING STANDARDS, UNLESS NOTED OTHERWISE.
3. REINFORCING STEEL SUPPORT DEVICES SHALL BE IN ACCORDANCE WITH THE CRSI MANUAL OF STANDARD PRACTICE.
4. UNSCHEDULED BEAMS, SLABS, COLUMNS, AND WALLS, SHALL HAVE REINFORCING STEEL DETAILED IN ACCORDANCE WITH THE FOLLOWING:
   A. MINIMUM LAP SPLICE FOR ALL REINFORCING BARS SHALL BE 48 TIMES THE BAR DIAMETER, UNLESS NOTED OTHERWISE.
   B. LAP TOP REINFORCING BARS AT MID SPAN
   C. LAP BOTTOM REINFORCING BARS AT SUPPORTS.
   D. LAP VERTICAL BARS IN WALLS AND COLUMNS AT FLOOR LINES ONLY, UNLESS NOTED OTHERWISE.
   E. PROVIDE CORNER BARS, OF SAME SIZE, FOR ALL HORIZONTAL BARS AT THE INSIDE AND OUTSIDE FACES OF INTERSECTING BEAMS OR WALLS.
5. PROVIDE MINIMUM (2) #4 x 8'-0" BARS AT 45° AT ALL REENTRANT CORNERS IN SLAB ON GRADE AND ELEVATED SLABS.
6. REINFORCING STEEL INTERRUPTED BY OPENINGS OR EMBEDDED ITEMS IN SLABS OR WALLS, SHALL BE COMPENSATED FOR BY REPLACING AN EQUAL AMOUNT OF REINFORCING BARS AT THE SIDES OF THE OPENING, PARALLEL TO UNINTERRUPTED STEEL. COMPENSATION STEEL SHALL EXTEND BEYOND THE EDGE OF OPENING OR EMBED A MINIMUM OF 48 TIMES THE BAR DIAMETER.
7. WELDING OF REINFORCING BARS IS NOT PERMITTED, AND HEAT SHALL NOT BE PERMITTED IN THE FABRICATION OR INSTALLATION OF REINFORCEMENT.
8. WELDED STEEL WIRE FABRIC USED FOR CONCRETE REINFORCING SHALL BE INSTALLED IN FLAT SHEETS, AND SHALL CONFORM TO ASTM A185.
9. MINIMUM CONCRETE COVERAGE FOR REINFORCING STEEL SHALL CONFORM TO THE FOLLOWING:
   A. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: 3"
   B. CONCRETE EXPOSED TO EARTH OR WEATHER:
      #6 BAR OR LARGER 2"
      #5 BAR OR SMALLER 1 1/2"
   C. CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT W/ GROUND:
      SLABS, WALLS, JOISTS 3/4"
      BEAMS, COLUMNS 1 1/2"

CONCRETE AND CMU ANCHORS

1. CONCRETE ANCHOR BOLT MATERIAL SHALL COMPLY WITH ASTM F1554 GRADE 36 UNO. ANCHOR BOLTS SHALL HAVE CUT THREADS AND SHALL BE FURNISHED WITH STD WASHER AND HEAVY HEX NUT.
2. EXPANSION ANCHORS SHALL BE HILTI KWIK BOLT III OR APPROVED EQUIVALENT.
3. ADHESIVE ANCHOR SYSTEM SHALL BE HILTI HIT-HY 150 MAX ADHESIVE ANCHOR OR APPROVED EQUIVALENT.
4. POST-INSTALLED ANCHORS SHALL BE INSTALLED PER MFR SPECIFICATIONS.
5. CONTRACTOR SHALL BE RESPONSIBLE FOR LOCATING ALL REINFORCING AND EMBEDDED ITEMS THROUGH NON-DESTRUCTIVE METHODS PRIOR TO POST-INSTALLED ANCHOR INSTALLATION. NO REINFORCEMENT OR EMBEDDED ITEMS SHALL BE CUT. POST-INSTALLED ANCHOR LOCATIONS SHALL BE RELOCATED WITH ENGINEERS APPROVAL WHERE CONFLICTS OCCUR.
6. POST INSTALLED ANCHORS IN CMU SHALL BE IN GROUTED CELLS.
7. POST-INSTALLED ANCHORS SHALL BE GALVANIZED.
8. FILL ABANDONED HOLES WITH EPOXY, FLEXIBLE JOINT SEALER OR GROUT.
9. INSTALLATION OF POST-INSTALLED ANCHORS SHALL BE CONTINUOUSLY INSPECTED BY THE TESTING AGENCY IN ACCORDANCE WITH THE GOVERNING BUILDING CODE.

SHEET INDEX

STRUCTURAL DRAWINGS

TDI PROJECT # 201-156

TEXAS DESIGN INTERESTS, LLC
AUSTIN • HOUSTON
6001 W. WILLIAM CANNON
BUILDING 2, SUITE 203-C
AUSTIN, TX 78749
(512) 301-3389 (o)
(512) 301-3348 (f)
COMMERCIAL/RESIDENTIAL - CIVIL & STRUCTURAL ENGINEERING

pi architects
3500 Jefferson St, Suite 303
Austin, TX 78731
P: (512) 231-1910
F: (512) 231-1950
www.piarch.com
architecture + master planning
interiors + landscape
architecture

JEFFREY E. CLOYD
License No. 30636
PROFESSIONAL ENGINEER
STATE OF LOUISIANA
3/31/16
FIRM REG. # 0597

WATERSTONE AT BATON ROUGE
ASSISTED LIVING & MEMORY CARE
SIEGEN LANE
BATON ROUGE, LA

ISSUE:
PERMIT SET

REVISIONS:

DATE: 3/31/2016
PROJECT NUMBER: 14017
SHEET TITLE: GENERAL NOTES
SHEET: S1.0